Simcoe County
District School Board

ADDENDUM # 2

Solicitation Name: New Alcona Elementary School – Submission A

Solicitation No.: 2026-13478T

Date of Issue: April 13, 2026

Closing Date: April 22, 2026

Please see the attached Addendum.

Brian Torrie, Purchasing Supervisor
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The following additions, deletions and amendments are hereby made a part of the Drawings and Specifications for the above mentioned Project:

A. STRUCTURAL

1. Structural Addendum 1 – 10 April 2026 – 4 pages

STRUCTURAL ADDENDUM #1
Project Name: Lake Simcoe PS - SCDSB
Project Number: 20231581

April 10, 2026

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via email: pweppler@swarchitects.ca

Please find enclosed Structural Addendum #1 for the above-referenced project.

If you have any questions regarding the above, please contact our office.

Regards,
Salas O'Brien Canada Inc.

Cory Gray, P.Eng
Structural Engineer
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1.0 REFERENCE STRUCTURAL DRAWING S0-01 (RE-ISSUED)

.1 Updated Engineered Fill Note 3.5/3.5a regarding layer below slab-on-grade. 200mm of 19mm clear stone is required, and the Granular ‘A’ alternative is now removed. See clouded area.

2.0 REFERENCE STRUCTURAL DRAWING S1-01 (RE-ISSUED)

.1 Added storm line invert elevation. Adjusted lower footing elevation to allow line to pass through foundation wall. See clouded area.

.2 Clarified underside of footing elevations at SDFs around grids Q and R. See clouded area.

END OF STRUCTURAL ADDENDUM #1

DESIGN CRITERIA NOTES:

1. GENERAL

1.1. THE STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2024 ONTARIO BUILDING CODE (OBC). ALL CODES, MANUALS, STANDARDS, AND SPECIFICATIONS REFERRED TO SHALL BE THE LATEST EDITIONS INCLUDING ALL REVISIONS AND ADDENDA AS REFERENCED IN THE OBC.

1.2 BUILDING IMPORTANCE CATEGORY (TABLE 4.1.2.1 OF OBC) IS: HIGH

1.3. MISCELLANEOUS METAL, PRECAST AND STAIR FABRICATORS SHALL:
- 1.3.1. PROVIDE SHOP DRAWINGS TO THE ARCHITECT PRIOR TO FABRICATION; STAMPED, SIGNED AND DATED BY A PROFESSIONAL ENGINEER.
- 1.3.2. DESIGN ALL GUARDS TO MEET LATERAL LOADS DESCRIBED IN OBC CLAUSE 4.1.5.14.
- 1.3.3. DESIGN ALL HANDRAILS TO MEET LOADS DESCRIBED IN OBC CLAUSE 4.1.5.14(7).
- 1.3.4. DESIGN ALL STAIRS TO SUPPORT A MINIMUM LIVE LOAD OF 4.8kPa.

1.4. ARCHITECTURAL FAÇADE (PRECAST AND INSULATED METAL PANEL, ETC.) FABRICATORS SHALL:
- 1.4.1. PROVIDE SHOP DRAWINGS TO THE ARCHITECT PRIOR TO FABRICATION, STAMPED, SIGNED AND DATED BY A PROFESSIONAL ENGINEER.
- 1.4.2. WHERE THE FAÇADE IS USED AS A GUARD AS MENTIONED ABOVE THE DESIGN OF ALL COMPONENTS AND CONNECTIONS SHALL MEET LATERAL LOADS DESCRIBED IN OBC CLAUSE 4.1.5.14.

1.5. IT IS THE RESPONSIBILITY OF THE CONTRACTOR WHO IS SUPPLYING AND INSTALLING NON-STRUCTURAL COMPONENTS AND EQUIPMENT, THAT ALL NON-STRUCTURAL ELEMENTS AND EQUIPMENT (AND THEIR CONNECTIONS TO THE STRUCTURE) LISTED IN TABLE 4.1.8.18 OF THE OBC ARE DESIGNED IN ACCORDANCE WITH CLAUSE 4.1.8.18.

2. FIRE SAFETY

2.1 REFER TO ARCHITECTURAL DRAWINGS FOR ALL FIRE SAFETY RATING REQUIREMENTS FOR STRUCTURAL COMPONENTS.
- 2.1.1. HOLLOW CORE SLABS SHALL BE DESIGNED TO MEET THE FIRE RATING AS NOTED ON THE ARCHITECTURAL DRAWINGS.

3. LATERAL LOADS ON STRUCTURE

3.1 WIND LOADING

3.1.1 WIND PARAMETERS
THE WIND LOADING PARAMETERS INDICATED BELOW HAVE BEEN CALCULATED IN ACCORDANCE TO CLAUSE 4.1.7 OF NBC 2020 AND COMMENTARY I - "WIND LOAD AND EFFECTS" OF USER'S GUIDE -NBC 2020 STRUCTURAL COMMENTARIES (PART 4 OF DIVISION B).
Ce and Cg HAVE BEEN COMPUTED ACCORDING TO THE [STATIC PROCEDURE FOR [OPEN TERRAIN],
IMPORTANCE FACTOR FOR WIND LOADING, Iw = 1.15
q(1/50) = 0.36 kPa
Cp VALUES PER CLAUSE 4.1.7.5 OR 4.1.7.6 OF NBC 2020 AND NBC 2020 USER'S GUIDE
CPi, VALUES PER CLAUSE 4.1.7.7 IN NBC 2020 AND NBC 2020 USER'S GUIDE.

3.1.2 BASE SHEARS
WIND BASE SHEAR =1660 kN (EAST-WEST DIRECTION)
WIND BASE SHEAR = 909 kN (NORTH-SOUTH DIRECTION)

3.1.3 IT IS THE RESPONSIBILITY OF THE SUBCONTRACTOR'S ENGINEER TO CALCULATE WIND PRESSURES ON NON-STRUCTURAL COMPONENTS (CLADDING, ROOFING, ETC.) BASED ON COMMENTARY I OF NBC 2020 USER'S GUIDE.

3.2 EARTHQUAKE LOADING

3.2.1 EARTHQUAKE PARAMETERS
IMPORTANCE FACTOR FOR EARTHQUAKE LOAD I_E =1.3
SITE DESIGNATION FOR SITE RESPONSE = Xc (REFER TO GEOTECHNICAL REPORT)
THE EARTHQUAKE SEISMIC CATEGORY AS PER OBC TABLE 4.1.8.5.-B = SC 2 ($I_ES(0.2)$ = 0.36)
Sa(0.2) = 0.216 Sa(5.0)= 0.0106
Sa(0.5) = 0.142 Sa(10.0) =0.00374
Sa(1.0) = 0.0809 PGA = 0.0923
Sa(2.0) = 0.0392
SFRS CONSISTS OF: CONVENTIONAL MASONRY SHEAR WALLS AND CORRESPONDING DUCTILITY-RELATED MODIFICATION FACTORS ARE:
Rd = 1.5 Ro = 1.5
METHOD OF LATERAL ANALYSIS: STATIC

3.2.2 BASE SHEARS
EARTHQUAKE BASE SHEAR = 2527 kN (EAST-WEST DIRECTION)
EARTHQUAKE BASE SHEAR = 2527 kN (NORTH-SOUTH DIRECTION)

3.2.3 STIFF ELEMENTS NOT PART OF SFRS SHALL BE SEPARATED FROM THE STRUCTURE AS PER OBC CLAUSE 4.1.8.3 (6a). EXAMPLES INCLUDE, BUT NOT LIMITED TO MASONRY PARTITIONS, BRICK VENEER, PRECAST CLADDING ETC. IT IS THE RESPONSIBILITY OF THE SUBCONTRACTOR TO PROVIDE SHOP DRAWINGS, STAMPED, SIGNED AND DATED BY A LICENSED PROFESSIONAL ENGINEER DEMONSTRATING COMPLIANCE. PROVIDE MINIMUM 25mm SEPARATION UNLESS NOTED OTHERWISE.

3.3 LATERAL FORCES ON FOUNDATION / RETAINING WALLS

3.3.1 WALLS RETAINING EARTH ARE DESIGNED TO SAFELY WITHSTAND HORIZONTAL EARTH PRESSURE (P IN kPa) AT A DEPTH (h IN m) GIVEN BY THE EQUATION BELOW.
P = K x (Wt.h+q)
Where: Ka = 0.31 Wt =21 kN/m^3
q = 12 kPa (SURFACE PRESSURE SURCHARGE)
h = DEPTH (m)

3.3.2 THE WALLS HAVE BEEN DESIGNED ASSUMING FREE DRAINING BACKFILL OR THE USE OF A DRAINAGE CORE TO PREVENT THE BUILD-UP OF HYDROSTATIC PRESSURE. (SEE TYPICAL DETAILS)

4. SUPERIMPOSED DEAD AND LIVE LOADS

4.1 SEE FLOOR/ROOF PLANS, NOTES, AND/OR LOADING SCHEDULES FOR DESIGN SUPERIMPOSED DEAD AND LIVE LOADING ASSUMPTIONS.

4.2 THE STRUCTURE HAS BEEN DESIGNED TO RESIST THE FOLLOWING VERTICAL CLADDING DEAD LOADS:

CURTAIN WALL	0.38 kPa.
BRICK VENEER	1.32 kPa.
METAL CLADDING	0.16kPa.

5. OTHER LOADS ON ROOFS

5.1 SNOW LOADING

5.1.1 THE SPECIFIED SNOW LOADING HAS BEEN CALCULATED BASED ON THE FOLLOWING PARAMETERS, MODIFIED TAKING INTO ACCOUNT FACTORS SUCH AS ROOF SIZE, WIND EXPOSURE, SLOPE AND SNOW DRIFTS:
IMPORTANCE FACTOR FOR SNOW LOAD Is =1.15
GROUND SNOW LOAD, Ss =2.5 kPa
ASSOCIATED RAIN LOAD, Sr = 0.4 kPa

5.1.2 SEE ROOF PLANS AND ACCUMULATED SNOW LOADING PLANS FOR LOCALIZED INCREASES AND DRIFTS CALCULATED IN ACCORDANCE WITH PART 4.1.6 OF NBC 2020 AND NBC 2020 STRUCTURAL COMMENTARIES.

5.2 RAIN LOADING

5.2.2 THE BUILDING ROOF STRUCTURE HAS BEEN DESIGNED ON THE ASSUMPTION THAT FLOW CONTROL ROOF DRAINS SATISFY ALL REQUIREMENTS OF THE 2020 NATIONAL PLUMBING CODE OF CANADA.

5.2.2 THE TOTAL RAIN LOAD ASSOCIATED WITH THE ONE DAY (24 HR) RAINFALL, IN ACCORDANCE WITH CLAUSE 4.1.6.4.(1) OF NBC 2020 AND COMMENTARY H IN USER'S GUIDE - NBC 2020 STRUCTURAL COMMENTARIES (PART 4 OF DIVISION B) IS: ONE DAY RAIN (1/50) = 97mm

5.3 WIND UPLIFT

5.3.1 SEE ROOF PLANS, NOTES OR ROOF UPLIFT LOADING SCHEDULE FOR DESIGN LOAD VALUES FOR WIND UPLIFT. THE VALUES PRESENTED ON THESE DRAWINGS ARE UNFACTORED LOADS.

5.3.2 UPLIFT PRESSURES HAVE BEEN CALCULATED IN ACCORDANCE WITH CLAUSE 4.1.7.5 OR 4.1.7.6 OF NBC 2020.

5.3.3 FOR WIND PARAMETERS REFER TO SECTION 3.1 - WIND LOADING.

6. SERVICEABILITY

6.1 TYPICAL HORIZONTAL ELEMENTS (NOT SUPPORTING CLADDING) HAVE BEEN DESIGNED SO THAT THE CALCULATED DEFLECTIONS DO NOT EXCEED THE FOLLOWING LIMITS:

MEMBER TYPE	DEFLECTION COMPONENT	DEFLECTION LIMIT
STRUCTURAL STEEL MEMBERS		
FLOOR MEMBERS	LIVE	L/360
FLOOR MEMBERS	TOTAL = DEAD + LIVE (MINUS CAMBER)	L/300
ROOF MEMBERS SUPPORTING CONSTRUCTION AND FINISHES SUSCEPTIBLE TO CRACKING	LIVE OR SNOW LOAD	L/360
ROOF MEMBERS SUPPORTING CONSTRUCTION AND FINISHES NOT SUSCEPTIBLE TO CRACKING	LIVE OR SNOW LOAD	L/300
ROOF MEMBERS	TOTAL = DEAD + LIVE (MINUS CAMBER)	L/240

6.2 PERIMETER OR SPANDREL ELEMENTS SUPPORTING CLADDING HAVE BEEN DESIGNED FOR AN ALLOWABLE DEFLECTION OF 1/480, OR [25] mm, WHICHEVER IS LESS.

6.3 THE BUILDING STRUCTURE HAS BEEN DESIGNED SUCH THAT THE TOTAL DRIFT PER STOREY UNDER SERVICE WIND AND GRAVITY LOADS DOES NOT EXCEED [1/500] OF THE STOREY HEIGHT.

6.4 THE BUILDING STRUCTURE HAS BEEN DESIGNED SUCH THAT THE CALCULATED INTERSTOREY DEFLECTIONS DUE TO SEISMIC LOADS PER OBC 4.1.8.13 DO NOT EXCEED [0.02] OF THE STOREY HEIGHT.

FLOOR LOADING SCHEDULE

LOADING / ROOF USE	SUPERIMPOSED DEAD LOAD (kPa)	LIVE LOAD (kPa)
CLASSROOM *	1.54	2.4
CORRIDOR / STAIRS *	1.8	4.8
LOW ROOF AREA *	1.06	2.76 +ASL
MECHANICAL ROOM *	3.6	6.0
STORAGE ROOMS *	1.54	4.8

* - **AS NOTED ON SCHEDULE**
HOLLOWCORE SLABS SHALL BE DESIGNED TO RESIST THE UNIFORM LOADING ABOVE AND IN ADDITION SHALL SUPPORT MASONRY PARTITION LOADS.
(CO-ORDINATE WITH ARCHITECTURAL DRAWINGS)
140 PARTITION, 6.7kN/m AND 190 PARTITIONS, 8.4kN/m

ROOF LOADING SCHEDULE

LOADING / ROOF USE	SUPERIMPOSED DEAD LOAD (kPa)	SNOW LOAD (kPa)
GENERAL ROOF	1.11	2.76+ASL
CORRIDOR ROOF	1.26	2.76
MECHANICAL ROOF	1.76	2.76
PRECAST SLAB ROOF	1.06	2.76+ASL

IN ADDITION TO UNIFORM LOADING SHOWN, REFER TO ROOF PLAN FOR ADDITIONAL LOADING FOR ACCUMULATED SNOW LOADS (ASL) AS SHOWN, AND FOR POINT LOADS OF BRACING AND MECHANICAL EQUIPMENT.

IN ADDITION TO UNIFORM LOADING SHOWN, DESIGN JOISTS AND PRECAST SLAB FOR ANY CONCENTRATED LOADS RESULTING FROM MECHANICAL PIPING OR AS A MINIMUM, DESIGN FOR POINT LOAD OF 2kN AT ANY LOCATION.

NOTES:
1. ROOFING SINGLE PLY = 0.72 kPa HAS BEEN INCLUDED IN THE ABOVE TABLE
2. PAVER LOADING, IF APPLICABLE. SHALL BE COORDINATED WITH THE EXTENTS SHOWN IN THE ARCHITECTURAL DDRAWINGS. WEIGHTS SHALL BE COORDINATED WITH THE SPECIFICATIONS AND FINAL PRODUCT SHOP DRAWINGS.

CONCRETE MIX SCHEDULE

EXPOSURE	ELEMENT	MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS (MPa^1)	EXPOSURE CLASSIFICATION	NOTES
GENERAL NON-EXPOSED CONCRETE (i.e., NOT EXPOSED TO CHLORIDES NOR FREEZE AND THAW)	FOOTINGS	25	N	
	COLUMNS	25	N	
	WALLS (NOT IDENTIFIED AS SHEAR WALLS)	25	N	
	SLAB ON GRADE ^{2}	25	N	
	SLAB ON METAL DECK	25	N	
	LEAN MIX	5	N	
	FLOATING SLABS	25	N	
	HOUSEKEEPING PADS	25	N	
	TOPPINGS	25	N	
	UNSHRINKABLE FILL	0.4 MAX.	N	
EXTERIOR EXPOSED CONCRETE EXCLUDING PARKING (i.e., EXPOSED TO FREEZE AND THAW BUT NOT CHLORIDES)	FOUNDATION/RETAINING WALLS	25	F-2	
	SLAB ON GRADE ^{2}, SIDEWALKS, LANDSCAPE CURBS	32	C-2	
	SLAB ON METAL DECK	25	F-2	
AREAS EXPOSED TO FREEZE AND THAW AND CHLORIDES	FOOTINGS	35	C-1	
	FOUNDATION / RETAINING WALLS	35	C-1	
	STAIR / RAMPS	35	C-1	
	SLAB ON GRADE ^{2}- UNREINFORCED OR FIBRE	32	C-2	
	SLAB ON GRADE ^{2}- STEEL OR WWF REINFORCED	35	C-1	
	FROST SLABS	35	C-1	
GROUT	MASONRY FILL/BOND BEAMS	15 (FINE GROUT)		CONFORM TO REQUIREMENTS OF CSA A179

1) STRENGTH SPECIFIED AT 28 DAYS U.N.O IN DRAWINGS AND SCHEDULES.
2) REINFORCED WITH SYNTHETIC FIBERS ADDED AT BATCHING PLANT - SEE SPECIFICATIONS
3) MAXIMUM 10mm AGGREGATE

SECOND FLOOR LINTEL SCHEDULE

REFER TO LINTEL NOTES A06 ON TYPICAL DETAIL DRAWINGS
SEE ALSO SPECIFICATION

MARK	MATERIAL	TYPE	REMARKS
2L1	2-L89x89x7.9	┘└	
2L2	2-L152x89x7.9 (LLV)	┘└	
2L3	W200x42 + 170x8mm BOTTOM PLATE	I	WP1 E.E. ** LINTEL CENTRED ON WALL
2L4	HSS 203x102x6.4 + 360x8mm BOTTOM PLATE	□	WP1 E.E. ** SEE DL1/S0-01
2L5	W410X39 + 170x8mm BOTTOM PLATE	I	WP1 E.E. ** LINTEL CENTRED ON WALL
2L6	W200X27 + 270x8mm BOTTOM PLATE	I	WP3 E.E. ** LINTEL CENTRED ON WALL
2L7	HSS 203x102x9.5 + 360x8mm BOTTOM PLATE	□	WP1 E.E. ** SEE DL1/S0-01
2L8	W200X27 + 270x8mm BOTTOM PLATE	I	WP3 E.E. ** LINTEL CENTRED ON WALL
2L9	HSS 203x152x6.4 + 410x8mm BOTTOM PLATE	□	WP2 E.E. ** SEE DL2/S0-01
2L10	HSS 203x152x6.4 + 460x8mm BOTTOM PLATE	□	WP3 E.E. ** SEE DL2/S0-01
2L11			

LENGTHS MAY VARY

ALL EXTERIOR LINTELS SUPPORTING FACE BRICK TO BE GALVANIZED
Tf = 10kN.M TORSION CONNECTION

** WELDED TO HSS EACH END.

ROOF LINTEL SCHEDULE

lake simcoe

REFER TO LINTEL NOTES A06 ON TYPICAL DETAIL DRAWINGS
SEE ALSO SPECIFICATION

MARK	MATERIAL	TYPE	REMARKS
RL1	2-L89x89x7.9	┘└	
RL2	W200x27 + 170x8mm BOTTOM PLATE	I	WP1 E.E. ** LINTEL CENTRED ON WALL
RL3	HSS 203x102x6.4 + 360x8mm BOTTOM PLATE	□	WP1 E.E. ** SEE DL1/S0-01
RL4	2-L102x102x7.9	┘└	
RL5	HSS 203x203x6.4 + 460x8mm BOTTOM PLATE	□	WP3 E.E. ** SEE DL3/S0-01
RL6	W200x42 + 170x8mm BOTTOM PLATE	I	WP1 E.E. ** LINTEL CENTRED ON WALL
RL7	W410X39 + 170x8mm BOTTOM PLATE	I	WP1 E.E. ** LINTEL CENTRED ON WALL
RL8	W200X27 + 220x8mm BOTTOM PLATE	I	WP2 E.E. ** LINTEL CENTRED ON WALL

LENGTHS MAY VARY

Tf = 10kN.M TORSION CONNECTION
ALL EXTERIOR LINTELS SUPPORTING FACE BRICK TO BE GALVANIZED

** WELDED TO HSS EACH END.

MECHANICAL LINTEL SCHEDULE

LINTELS IN LOAD BEARING WALLS OVER MECHANICAL DUCTS ETC.

WALL THICKNESS	CLEAR SPAN	MATERIAL	TYPE	NOTES
190	200-550	175x8 PLATE	—	
190	550-1220	2-L90x90x6	┘└	
240	200-550	225x8 PLATE	—	
240	550-1220	2-L100x100x8	┘└	
290	200-550	275x8 PLATE	—	
290	550-1220	3-L90x90x6	┘└ └	
190 + 90	200-550	175x8 PLATE + 80x8 PLATE	— —	CAVITY WALLS EXTERIOR ANGLES & PLATES GALVANIZED UNLESS NOTED
190 + 90	550-1220	2-L90x90x6 + 1-L90x90x6	┘└ └	
240 + 90	200-550	225x8 PLATE + 80x8 PLATE	— —	
240 + 90	550-1220	2-L100x100x8 + 1-L90x90x6	┘└ └	
290 + 90	200-550	275x8 PLATE + 80x8 PLATE	— —	
290 + 90	550-1220	3-L90x90x6 + 1-L90x90x6	┘└└ └	

1 FOR LINTELS MARKED ML ON DRAWINGS.
2 FOR SPANS LESS THAN 200mm - NO LINTEL REQUIRED.
3 FOR SPANS GREATER THAN 1200mm, SEE PLANS AND MAIN LINTEL SCHEDULE.

WHILE EVERY EFFORT HAS BEEN MADE TO SHOW ALL LINTELS WHICH OCCUR IN LOAD BEARING MASONRY WALLS, IT IS THE CONTRACTORS RESPONSIBILITY TO ENSURE THAT THE CORRECT SIZES AND QUANTITY OF LINTELS ARE PROVIDED

LINTELS IN NON-LOAD BEARING WALLS AND PARTITIONS ARE GENERALLY NOT SHOWN ON THE DRAWINGS. ALL SUCH LINTELS SHALL BE PROVIDED AS REQUIRED AND SHALL CONFORM TO THE NOTES & TYPICAL DETAILS ON THE STRUCTURAL DRAWINGS

PROVIDE MECHANICAL LINTELS IN ACCORDANCE WITH TYPICAL DETAILS AND NOTES FOR ALL DUCTS AND PIPES PASSING THROUGH MASONRY WALLS

SECOND FLOOR / LOW ROOF FRAMING PLAN

1. TOP OF PRECAST SLAB TO BE -50mm BELOW FINISHED FLOOR DATUM, ELEVATION +4000mm EXCEPT AS CROSSED AND NOTED. T.O.S = TOP OF SLAB.
2. TOPS OF STEEL BEAMS TO BE AT UNDERSIDE OF PRECAST SLABS (-304mm) EXCEPT AS SHOWN THUS ON ± PLAN
3. REFER TO LOADING SCHEDULE ON THIS DRAWING. HOLLOWCORE SLABS SHALL BE DESIGNED TO SUPPORT THE SPECIFIED DEAD AND LIVE LOADS AND IN ADDITION SHALL SUPPORT MASONRY PARTITION LOADS. (CO-ORDINATE WITH ARCHITECTURAL DRAWINGS AND LOADING SCHEDULE).
4. HOLLOWCORE SLABS SHALL HAVE A FIRE RATING OF 2 HOURS.
5. SUBMIT DETAILS FOR ALL OPENINGS, OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS, TO THE STRUCTURAL CONSULTANT FOR REVIEW.
6. LOCATIONS OF MECHANICAL EQUIPMENT AND MECHANICAL EQUIPMENT LOADS ARE TO BE CONFIRMED BY THE MECHANICAL CONTRACTOR BEFORE PROCEEDING.
7. AN INDEPENDENT INSPECTION AND TESTING COMPANY IS TO INSPECT STRUCTURAL STEEL IN THE SHOP AND IN THE FIELD FOR WELDING, CONNECTIONS, BOLT TORQUES, AND GENERAL CONFORMANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS.
8. NON - LOAD BEARING PARTITIONS SHALL BE A MINIMUM OF 25 mm CLEAR OF STRUCTURE.
9. WALL PLATES (WP) SHALL HAVE LAST DIMENSION PARALLEL TO BEAM OR JOIST WEB. SEE WALL PLATES SCHEDULE.
10. SEE LINTEL SCHEDULE ON THIS DRAWING.
11. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
12. SEE TYPICAL NOTES, TYPICAL DETAILS, COLUMN SCHEDULE AND ALL OTHER DRAWINGS.
13. PROVIDE A MAXIMUM 50mm BONDED, NON - COMPOSITE,TOPPING SLAB ON PRECAST HOLLOWCORE SLABS, UNLESS OTHERWISE NOTED. TOP SURFACE IS TO BE FLAT (SEE SPECIFICATION FOR PREPARATION)
14. PARTITIONS ABOVE PRECAST HOLLOWCORE SLABS ARE TO BE CONSTRUCTED BEFORE TOPPING IS PLACED.
15. FOR CONCRETE BASE AT LOCKERS, SEE ARCHITECTURAL DRAWINGS.
16. PROVIDE 100 mm HOUSE KEEPING PADS IN MECHANICAL ROOM (SEE MECHANICAL DRAWINGS FOR SIZE AND LOCATION) REINF: 10m@300 EACH WAY.
17. PROVIDE MIN 100X100 CURBS AROUND ALL OPENINGS AND AT DOORS IN MECHANICAL ROOM FLOOR. REINF: 10M@300 DOWELS FROM BASE SLAB + 1-10M CONT. (REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS).
18. REFER TO ARCHITECTURAL DRAWINGS FOR FIRE PROOFING OF BEAMS AND COLUMNS.
19. REFER TO CONCRETE MIX SCHEDULE ON THIS DRAWING.
20. IF ANY ASPECT OF THE PRECAST HOLLOWCORE SLAB CONSTRUCTION, PROPOSED BY THE CONTRACTOR FOR USE ON THIS PROJECT, VARIES FROM THAT SHOWN ON THE TENDER DRAWINGS AND THE VARIATION REQUIRES THE RE-DESIGN OF THE BASE BUILDING ELEMENTS AND/OR THE REVISION TO OR ADDITION OF MATERIALS TO ACCOMMODATE SUCH VARIANCE AND THE OWNER, THE ARCHITECT AND THE STRUCTURAL CONSULTANT ARE IN AGREEMENT WITH THE PROPOSED VARIATION, THEN THE CONTRACTOR REQUESTING THE VARIATION SHALL PAY FOR ALL EXTRA COSTS, INCLUDING RE-DESIGN, ASSOCIATED WITH THE CHANGE.

ROOF FRAMING PLAN NOTES

1. UNDERSIDE OF ROOF DECK TO BE0.0mm BELOW ROOF DATUM ELEVATION +7900mm EXCEPT AS NOTED ON PLAN U.O.D. = UNDERSIDE OF DECK.
2. SEE ARCH. DRAWINGS FOR ALL ROOF SLOPE AND DRAINAGE INTENT.
3. TOP OF STEEL BEAMS SUPPORTING STEEL DECK ARE 0.0mm BELOW U.O.D. TOP OF STEEL BEAM SUPPORTING JOISTS ARE 150mm BELOW U.O.D EXCEPT AS SHOWN THUS ± ON PLAN.TOPS OF STEEL BEAMS SUPPORTING PRECAST ROOF SLAB TO BE AT UNDERSIDE OF PRECAST SLABS (-216mm) EXCEPT AS SHOWN THUS ± ON PLAN
4. OWSJ SHOES TO BE 150mm DEEP UNLESS NOTED.
5. FOR LOADING SEE ROOF LOADING SCHEDULE ON THIS DRAWINGS.
6. OPEN WEB STEEL JOISTS SHALL BE DESIGNED TO SUPPORT TOTAL DEAD, SNOW AND SPACING AS SHOWN ON PLAN. LOADS. IN ADDITION, OWSJ SHALL BE DESIGNED FOR ADDITIONAL LOADS SHOWN ON PLAN, ACCUMULATED SNOW DRIFT LOADS (ASL) AND FOR POINT LOADS OF BRACING AND MECHANICAL EQUIPMENT, IN EXCESS OF 2.0kN PER JOIST.
7. JOISTS AND BEARING ANCHORAGES SHALL BE DESIGNED TO RESIST UPLIFT DUE TO WIND AS REQUIRED BY THE ONTARIO BUILDING CODE AND IN NO CASE LESS THAN THE GREATER OF THOSE INDICATED ON THE WIND UPLIFT KEY PLAN.
8. OWSJ SHALL BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER, SHOP DRAWINGS AND CALCULATIONS BEARING THE STAMP AND SIGNATURE OF THE PROFESSIONAL ENGINEER RESPONSIBLE FOR THE DESIGN SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION AND ERECTION.
9. PROVIDE CONTINUOUS TOP AND BOTTOM CHORD BRIDGING FOR OWSJ IN ACCORDANCE WITH CSA-S16.1 IN NO CASE LESS THAN AS SHOWN ON PLAN.
10. PROVIDE BOTTOM CHORD BRACING FOR ALL JOISTS NEAR THE FIRST BOTTOM CHORD PANEL POINT. BRIDGING CANNOT BE CONSIDERED AS BRACING.
11. "TJ" NOTED AGAINST JOIST DENOTES "TIE JOIST" I.E EXTEND BOTTOM CHORD AND CONNECT TO SUPPORT. TIE JOIST TO BE DESIGNED FOR 1% OF THE COLUMN AXIAL LOAD. SEE COLUMN SCHEDULE.
12. LIVE LOAD DEFLECTION OF ROOF JOISTS SHALL NOT EXCEED 1/240 OF SPAN UNLESS OTHERWISE NOTED.
13. STEEL ROOF DECK SHALL BE DESIGNED TO SUPPORT SPECIFIED TOTAL DEAD AND LIVE LOADS. MINIMUM BASE NOMINAL THICKNESS (BNT) OF STEEL DECK SHALL BE 0.76 MM.
14. NO HANGERS OR BRACKETS SUPPORTING MECHANICAL EQUIPMENT OR PIPING SHALL BE HUNG FROM ROOF DECK.
15. STEEL ROOF DECK SHALL BE INSTALLED FOR DIAPHRAGM ACTION IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE CANADIAN SHEET STEEL BUILDING INSTITUTE AND TYPICAL NOTES.
16. LOCATION OF MECHANICAL EQUIPMENT AND MECHANICAL EQUIPMENT LOADS ARE TO BE CONFIRMED BY MECHANICAL CONTRACTOR BEFORE STRUCTURAL STEEL IS FABRICATED. REFER TO MECHANICAL DRAWINGS. UNLESS OTHERWISE APPROVED, MECHANICAL EQUIPMENT AND PIPING MUST BE HUNG FROM OWSJ PANEL POINTS AND HANGER SPACING SHALL NOT EXCEED 3.0 M.
17. FRAME ALL ROOF OPENINGS AND MECHANICAL UNITS AS SHOWN ON TYPICAL DETAILS UNLESS NOTED.
18. SUBMIT DETAILS TO STRUCTURAL CONSULTANT FOR REVIEW FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON STRUCTURAL DRAWINGS.
19. "M" DENOTES MOMENT CONNECTION. FACTORED MOMENT AS SPECIFIED ON DRAWINGS IS IN kN METRES.
20. MOMENT CONNECTIONS SHALL BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER. SHOP DRAWINGS AND CALCULATIONS BEARING THE STAMP AND SIGNATURE OF THE LICENSED PROFESSIONAL ENGINEER RESPONSIBLE FOR THE DESIGN SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION AND ERECTION.
21. AN INDEPENDENT INSPECTION AND TESTING COMPANY IS TO INSPECT STRUCTURAL STEEL AND STEEL DECK IN THE SHOP AND IN THE FIELD FOR WELDING, CONNECTIONS, BOLT TORQUES, AND GENERAL CONFORMANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS.
22. NON-LOAD BEARING PARTITIONS SHALL BE A MINIMUM OF 25 mm CLEAR OF STRUCTURE.
23. WALL PLATES (WP) SHALL HAVE LAST DIMENSION PARALLEL TO BEAM OR JOIST WEB. SEE SCHEDULE ON DRAWINGS.
24. SEE ROOF LINTEL SCHEDULE ON THIS DRAWING .
25. REFER TO GENERAL NOTES AND SPECIFICATION FOR GRADES OF STRUCTURAL STEEL AND STEEL DECK.
26. SEE TYPICAL NOTES, TYPICAL DETAILS, COLUMN AND FOOTING SCHEDULES AND ALL OTHER DRAWINGS.
27. FOR LOCATION OF ROOF ANCHORS AND DAVIT SUPPORTS, REFER TO ARCHITECTURAL DRAWINGS. REFER TO TYPICAL DETAILS FOR CONNECTION DETAILS. SUBMIT SHOP DRAWINGS FOR REVIEW AND COORDINATION.
28. PRECAST SLABS TO BE DESIGNED TO SUPPORT SPECIFIED DEAD AND SNOW LOADS.
29. PRECAST SLABS SHALL HAVE A FIRE RATING OF 2 HOURS.
30. IF ANY ASPECT OF THE PRECAST HOLLOWCORE SLAB CONSTRUCTION, PROPOSED BY THE CONTRACTOR FOR USE ON THIS PROJECT, VARIES FROM THAT SHOWN ON THE TENDER DRAWINGS AND THE VARIATION REQUIRES THE RE-DESIGN OF THE BASE BUILDING ELEMENTS AND/OR THE REVISION TO OR ADDITION OF MATERIALS TO ACCOMMODATE SUCH VARIANCE AND THE OWNER, THE ARCHITECT AND THE STRUCTURAL CONSULTANT ARE IN AGREEMENT WITH THE PROPOSED VARIATION, THEN THE CONTRACTOR REQUESTING THE VARIATION SHALL PAY FOR ALL EXTRA COSTS, INCLUDING RE-DESIGN, ASSOCIATED WITH THE CHANGE.

WALL PLATE SCHEDULE

(LAST DIMENSION PARALLEL TO WEB)

MARK	MATERIAL	REMARKS
WP1	180x15x180	(2)13 Ø A.BOLTS x 150 LG. ⊓
WP2	200x15x200	(2)13 Ø A.BOLTS x 150 LG. ⊓
WP3	250x20x250	(2)13 Ø A.BOLTS x 150 LG. ⊓
WP4	400x30x180 + 25mm Ø ROCKER BAR	(2)13 Ø A.BOLTS x 150 LG. ⊓

MASONRY CORE FILL SCHEDULE

M20_2024

MASONRY CORE FILL SCHEDULE

TYPE	SIZE	REINF	REMARKS
C1	t x 400	2-15 VERT. CONT.	
C2	t x 600	3-15 VERT. CONT.	
C3	t x 800	4-15 VERT. CONT.	
C4	t x 400 x 400	4-15 VERT. CONT.	

t DENOTES THE WALL THICKNESS

MASONRY CORE FILLS NOTES:

1. PROVIDE CORE FILLS AS NOTED ON PLAN AND PROVIDE REINFORCEMENT AS SHOWN IN SCHEDULE.
2. CORE FILLS EXTEND FULL HEIGHT OF WALL, FLOOR TO FLOOR UNLESS NOTED.
3. INSTALL ALL REINFORCEMENT FULL HEIGHT BETWEEN FLOORS AND GROUT CORE SOLID FULL HEIGHT BETWEEN FLOORS UNLESS NOTED.
4. WHERE CORE FILL CONTINUES TO NEXT FLOOR ABOVE, EXTEND INDICATED VERTICAL REINFORCEMENT TO PROVIDE SPECIFIED CLASS "B" TENSION LAP SPLICE WITH REINFORCEMENT OF CORE ABOVE. WHERE MASONRY WALLS START ON TOP OF STEEL BEAMS, PROVIDE WELDABLE REINFORCING DOWELS TO MATCH REINFORCING NOTED IN THIS SCHEDULE, OR EQUIVALENT D2L DEFORMED BAR ANCHORS
5. PROVIDE 15M DOWELS IN FOUNDATION WALLS FOR ALL WALL REINFORCEMENT UNLESS NOTED OTHERWISE.
6. REFER TO M04 FOR LAP LENGTHS FOR VERTICAL BARS AND DOWELS.
7. REFER TO CORE FILLS SCHEDULE FOR DETAILS AND REINFORCEMENT.
8. PROVIDE CORE FILL C1 AT EACH SIDE OF OPENINGS U/N OTHERWISE NOTED ON PLANS AND/OR SECTIONS.
   - a) PROVIDE C1 AT UNSUPPORTED ENDS OF WALLS U/N.
   - b) PROVIDE C1 AT EACH SIDE OF MOVEMENT JOINTS U/N.
9. PROVIDE CORE FILL C4 AT ALL WALL CORNERS U/N OTHERWISE IN PLANS AND/OR SECTIONS.
10. PROVIDE RS OR VS SERIES MOVEMENT JOINT BY BLOK-LOK OR EQUIVALENT FOR ALL VERTICAL MOVEMENT JOINTS IN EXTERIOR MASONRY WALLS EXCEEDING 4m IN HEIGHT.
11. REINFORCE ALL MASONRY SILLS, INTERIOR AND EXTERIOR, AS PER THE REINFORCING INDICATED IN THIS SCHEDULE. GROUT TOP TWO COURSES OF ALL SILLS SOLID. FULLY GROUT ALL EXTERIOR SILLS.
12. 190mm MASONRY WALL REINFORCING - 15M@ 800 O/C.
    240mm MASONRY WALL REINFORCING - 15M@ 600 O/C.
    290mm MASONRY WALL REINFORCING - 2-15M@ 200 O/C

ENGINEERED FILL NOTES:

1. GENERAL

1.1. THE FOLLOWING ARE MINIMUM REQUIREMENTS FOR PLACING ENGINEERED FILL WITHIN THE BOUNDARIES OF THE BUILDING ENVELOPE AND EXTENDING BEYOND PERIMETER OF THE BUILDING FOUNDATIONS BY A MIN. OF **1000mm** AND SLOPING DOWNWARD TO THE SUB-GRADE, IN ALL DIRECTIONS, AT 45°.

1.2. PRIOR TO THE START OF CONSTRUCTION THE CONTRACTOR SHALL CONVENE A MEETING TO BE ATTENDED BY AT LEAST THE ARCHITECT, STEPHENSON ENGINEERING, THE SOIL CONSULTANT, THE GENERAL CONTRACTOR, AND THE EXCAVATION AND BACKFILLING CONTRACTOR. THE PURPOSE OF THIS MEETING IS TO ENSURE THAT ALL PARTIES UNDERSTAND THE REQUIREMENTS OF THE CONTRACT DOCUMENTS AND TO DISCUSS PROCEDURES, TIMING, MATERIALS AND TESTING, ETC.

1.3. REFER ALSO TO THE SPECIFICATION, THE SOIL REPORT AND DIAGRAMMATIC SECTION.

2. MATERIALS

2.1. ALL MATERIAL TO BE USED AS FILL MUST BE IMPORTED GRANULAR 'B' MATERIAL AS APPROVED BY THE SOIL CONSULTANT.

2.2. THE LAYER IMMEDIATELY BELOW THE SLAB-ON-GRADE SHALL BE A MIN. OF **150mm, 19mm CLEAR STONE.**

3. EXECUTION

3.1. REMOVE AND DISPOSE OF ALL **EXISTING ORGANIC MATERIAL, FILL, AND CONTAMINATED MATERIAL** DOWN TO NATURAL UNDISTURBED, UN-CONTAMINATED SUB-GRADE.

3.2. THE SUB-GRADE SHALL BE PROOF ROLLED WITH HEAVY VIBRATORY EQUIPMENT TO MIN. **98%** STANDARD PROCTOR MAXIMUM DRY DENSITY.

3.3. ANY LOOSE OR SOFT SPOT SHALL BE SUB-EXCAVATED AND BACKFILLED WITH APPROVED COMPACTED MATERIAL.

3.4. FILL REQUIRED TO RAISE THE GRADES SHALL COMPRISE OF APPROVED IMPORTED GRANULAR 'B' TYPE I MATERIAL PLACED IN SUCCESSIVE **200mm** LAYERS EACH COMPACTED TO AT LEAST **100%** STANDARD PROCTOR MAXIMUM DRY DENSITY. IN WET SUBGRADE AREAS, THE FIRST LIFT OF ENGINEERED FILL SHALL CONSIST OF 400 MM OF GRANULAR 'B' TYPE II

7

3.5. THE LAYER IMMEDIATELY BELOW THE SLAB-ON-GRADE SHOULD BE A **200mm MIN. LAYER OF 19mm CLEAR STONE ROLLED AND** COMPACTED TO A DENSE STATE. THE UPPER 50mm OF CLEAR STONE MAY BE REPLACED WITH 19mm CRUSHER RUN LIMESTONE. THE CLEAR STONE AND LIMESTONE SUBGRADE MUST BE SEPARATED BY A GEOTEXTILE SUCH AS TERRAFIX 270R OR APPROVED EQUIVALENT.

3.6. ALL PROCEDURES, EQUIPMENT AND MATERIALS SHALL BE APPROVED BY THE SOIL CONSULTANT WHO SHALL BE ENGAGED "FULL TIME" TO SUPERVISE THIS WORK.

3.7. CONDITIONS AS OUTLINED IN THE CONTRACT DOCUMENTS ARE ASSUMED AND ARE BASED UPON INFORMATION AVAILABLE AT THE TIME THAT THE DOCUMENTS WERE PREPARED.

3.8. THE SOIL CONSULTANT SHALL ISSUE, VIA "E-MAIL", DAILY REPORTS OF THE WORK.

3.9. IF ANY ASPECT OF THE ACTUAL WORK IS NOT AS ASSUMED, THEN THE SOIL CONSULTANT SHALL ADVISE THE ARCHITECT IMMEDIATELY, BY TELEPHONE, BEFORE PROCEEDING.

3.10.NOTE THAT THE EXISTING ON-SITE MATERIAL IS NOT SUITABLE FOR BACKFILLING OF TRENCHES, ETC., OR AGAINST FOUNDATION WALLS.

3.11.FOR AREAS UNDER DRIVEWAYS AND PARKING ETC., OUTSIDE BUILDING ENVELOPE, REFER TO SPECIFICATION AND SOIL REPORT.

FOUNDATION PLAN NOTES

1. ALL FOOTINGS SHALL BE FOUNDED ENGINEERED FILL CAPABLE OF SUSTAINING 150 kPa (SLS), 225 kPa (ULS)
2. REFER TO SOILS REPORT NO. 2302101 DATED OCTOBER 22 , 2024 AND UPDATED SEISMIC CLASS LETTER DATED AUGUST 7, 2025 BOTH PREPARED BY GEI CONSULTANT.
3. SOIL AT THE UNDERSIDE OF THE FOOTINGS IS TO BE INSPECTED AND APPROVED BY A REPRESENTATIVE OF A SOILS CONSULTANT BEFORE PLACING CONCRETE.
4. UNDERSIDE OF WALL FOOTINGS TO BE AT ELEVATIONS AS NOTED.
5. SLAB - ON - GRADE TO BE 100 mm THICK REINFORCED SYNTHETIC FIBRES. (SEE SPECIFICATION)
6. TOP OF SLAB - ON - GRADE TO BE AT FINISHED FLOOR DATUM ELEVATION, 245.55m EXCEPT AS CROSSED AND NOTED. TOS = TOP OF SLAB.
7. CENTRELINES OF COLUMNS, CAPS AND FOOTINGS ARE COINCIDENT UNLESS OTHERWISE NOTED.
8. PROVIDE SLAB DEPRESSIONS, OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS, AS REQUIRED BY THE ARCHITECTURAL AND MECHANICAL DRAWINGS AND SPECIFICATIONS.
9. SDF = STEP DOWN FOOTING.
10. UNLESS OTHERWISE NOTED, ALL WALL FOOTINGS TO BE 300 mm DEEP WITH 150 mm PROJECTIONS EACH SIDE.
11. FILL REQUIRED ON BOTH SIDES OF FOUNDATION WALLS SHALL BE PLACED AND COMPACTED SIMULTANEOUSLY ON BOTH SIDES TO EQUALIZE SOIL PRESSURE.
12. THE PROJECT SUPERINTENDENT MUST NOTIFY THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE, INCLUDING STRIP FOOTINGS.
13. SEE ALSO TYPICAL NOTES AND DETAILS.
14. SEE COLUMN SCHEDULE FOR COLUMNS, AND COLUMN FOOTINGS.
15. CONCRETE STRENGTHS - SEE CONCRETE SCHEDULE.
16. REFER TO SITE PREPARATION NOTES ON THIS DRAWING.

NOTE:
BOTTOM PLATES TO TERMINATE 10mm CLEAR OF SUPPORTING MASONRY-VERIFY WITH ARCH.

DL1
S0-01
NTS

NOTE:
BOTTOM PLATES TO TERMINATE 10mm CLEAR OF SUPPORTING MASONRY-VERIFY WITH ARCH.

DL2
S0-01
NTS

NOTE:
BOTTOM PLATES TO TERMINATE 10mm CLEAR OF SUPPORTING MASONRY-VERIFY WITH ARCH.

DL3
S0-01
NTS

TYPICAL DETAIL OF INFILL SLAB

SEE P.C. MANUFACTURER'S DRAWINGS FOR LOCATION AND QUANTITY (SEE ALSO SPECIFICATION) P.C. MANUFACTURER TO SUPPLY REINFORCING BARS AND CONCRETE

DAB1
S0-01
NTS

NOTE:
1. PARTITIONS ARE TO BE CONSTRUCTED PRIOR TO INSTALLATION OF TOPPING.
2. SEE ALSO FLOOR LOADING SCHEDULE.
3. IF BLOCK PARTITIONS ARE INSTALLED AFTER TOPPING IS POURED. GENERAL CONTRACTOR TO NOTE AND COORDINATE THE AFFECT OF POST PARTITION ERECTION ON THE CAMBER OF THE CORE SLABS. IN ADDITION, DOWELS MUST BE PROVIDED FROM CORESLAB INTO WALL ABOVE MIN. 10M@1200 U.N.O IN NOTES, SCHEDULES, TYPICAL DETAILS ETC.

ISSUED

No.	Description	Date
1	ISSUED FOR 60% CD	DEC 06,2024
2	ISSUED FOR 80% CD	DEC 20,2024
3	ISSUED FOR PROGRESS REVIEW	JAN 24,2025
4	ISSUED FOR PROGRESS REVIEW	JUL 21,2025
5	ISSUED FOR FINAL REVIEW	AUG 13, 2025
6	ISSUED FOR PERMIT / TENDER	MAR 27, 2026
7	ISSUED FOR ADDENDUM No.1	APR 10, 2026

THE CONTRACTOR SHALL CHECK ALL DIMENSIONS WITH THE LATEST ISSUE OF ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS. REPORT ANY DISCREPANCIES TO THE ARCHITECT BEFORE PROCEEDING WITH WORK.

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Stephenson Engineering, a company of Salas O'Brien

NEW ALCONA PUBLIC SCHOOL

CORM STREET

SIMCOE COUNTY
DISTRICT SCHOOL BOARD
INNISFIL, ON

26-04-10
J. C. GASDIA
100169574

Drawn by OH Date APRIL 2026
Checked by JG Scale As indicated

Drawing Title
DETAILS

Project number
20231581 S0-01

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FOOTING PROJECTION SCHEDULE	
FOOTINGS SHOWN THUS ON PLAN INDICATE 200mm FOOTING PROJECTION:- REINF. W/ 15@500B TRANS & 3-15B LONG	
FOOTINGS SHOWN THUS ON PLAN INDICATE 250mm FOOTING PROJECTION:- REINF. W/ 15@450B TRANS & 3-15B LONG	FOOTINGS SHOWN THUS ON PLAN INDICATE 250mm FOOTING PROJECTION:- REINF. W/ 15@350B TRANS & 3-15B LONG
FOOTINGS SHOWN THUS ON PLAN INDICATE 275mm FOOTING PROJECTION:- REINF. W/ 15@350B TRANS & 3-15B LONG	FOOTINGS SHOWN THUS ON PLAN INDICATE 300mm FOOTING PROJECTION:- REINF. W/ 15@400B TRANS & 3-15B LONG
FOOTINGS SHOWN THUS ON PLAN INDICATE 325mm FOOTING PROJECTION:- REINF. W/ 15@350B TRANS & 3-15B LONG	FOOTINGS SHOWN THUS ON PLAN INDICATE 350mm FOOTING PROJECTION:- REINF. W/ 15@300B TRANS & 3-15B LONG
FOOTINGS SHOWN THUS ON PLAN INDICATE 450mm FOOTING PROJECTION:- REINF. W/ 15@250B TRANS & 3-15B LONG	FOOTINGS SHOWN THUS ON PLAN INDICATE 500mm FOOTING PROJECTION:- REINF. W/ 15@250B TRANS & 3-15B LONG
NOTE: TRANS. REINF. HOOKED EACH END	

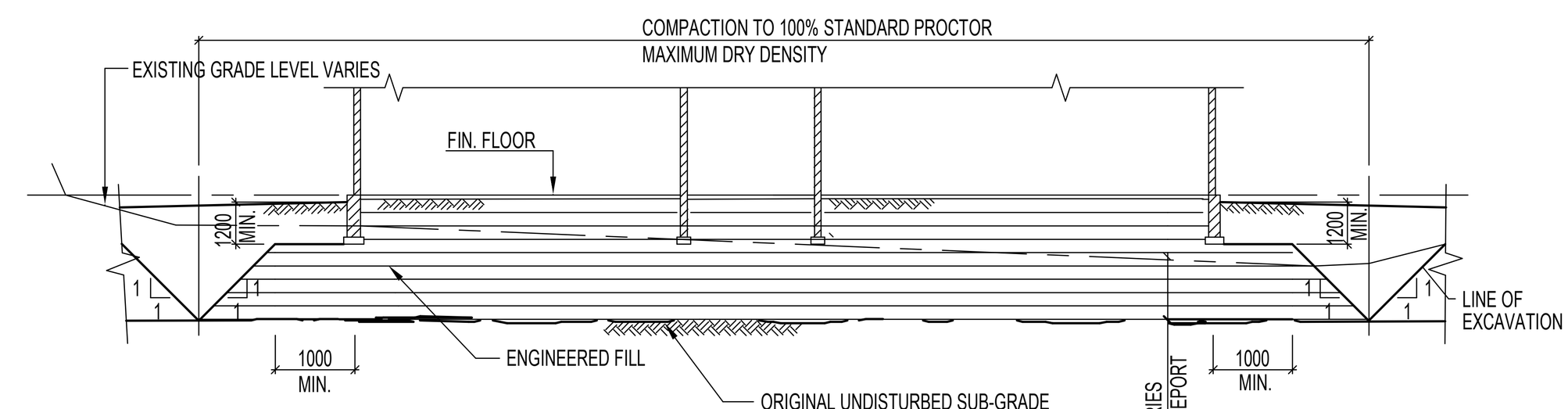
NOTE:
REFER TO DRAWING S0-01
FOR PLAN NOTES, DETAILS
AND MISC. SCHEDULES

LOWER ELEVATIONS AT UNDERSIDE OF COLUMN AND WALL FOOTINGS, WHERE REQUIRED, BUT NOT LIMITED TO STORM, SANITARY, WATER/FIRE LINES AND ELECTRICAL DUCT BANKS ETC.
THE MAXIMUM SLOPE FROM THE PIPE EXCAVATION TO THE UNDERSIDE OF ADJACENT FOOTING ELEVATIONS SHALL NOT EXCEED 7 VERTICAL TO 10 HORIZONTAL.

WHERE MECHANICAL SERVICE PIPES PASS THROUGH LOAD BEARING FOUNDATION WALLS, PROVIDE STEEL SLEEVES (MIN.50Ø) LARGER THAN PIPE (TYPICAL)

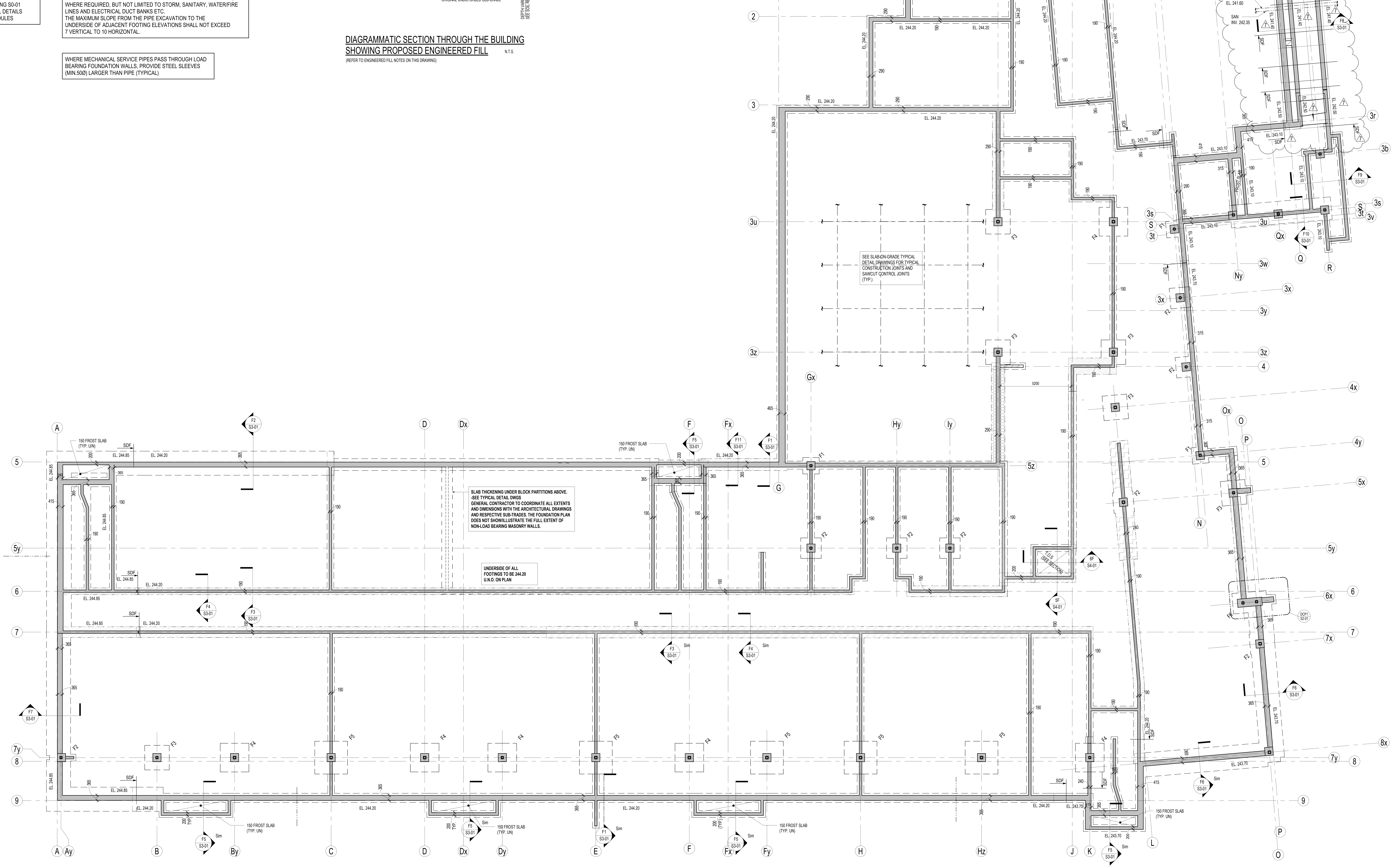
FOOTING SCHEDULE 1

FOOTING NUMBER	FOOTING LENGTH	FOOTING WIDTH	FOOTING THICKNESS	FOOTING REINF
F1	1200	1200	300	<varies>
F2	1500	1500	300	<varies>
F3	1800	1800	300	<varies>
F4	2100	2100	300	9-15M B EW
F5	2400	2400	350	13-15M B EW
F6	1500	2500	300	8-15M* 80 HOOK B EE



DIAGRAMMATIC SECTION THROUGH THE BUILDING SHOWING PROPOSED ENGINEERED FILL N.T.S.

(REFER TO ENGINEERED FILL NOTES ON THIS DRAWING)



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Drawing Title

FOUNDATION PLAN

Project number 20231581 S1-01

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